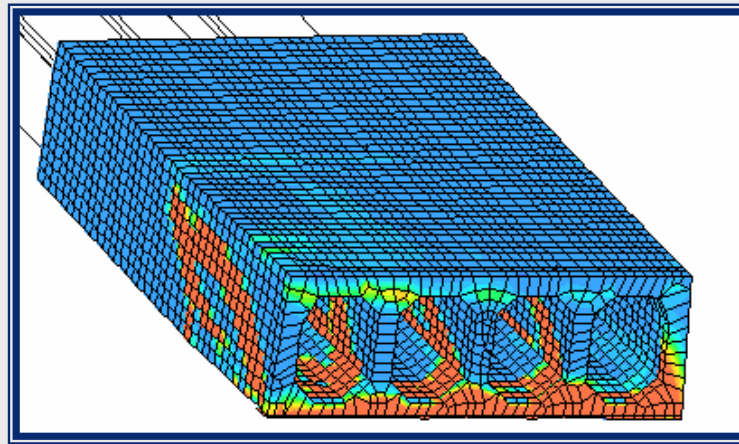


# Shear and Torsion in Hollow Core Slabs

## *Holcotos*

A European research project



*Financiers and collaboration partners*

- European Commission
- International Prestressed Hollow Core Association
- Bundesverband Spannbeton-Hohlplatten
- Castelo
- Consolis
- Echo
- A. Van Acker
- Strängbetong
- VTT
- Chalmers

# Shear and Torsion in Hollow Core Slabs



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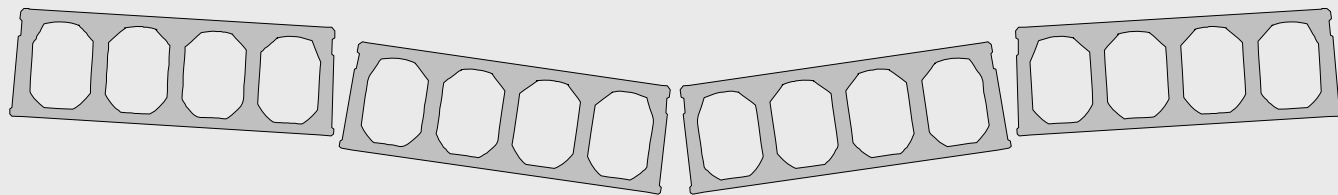


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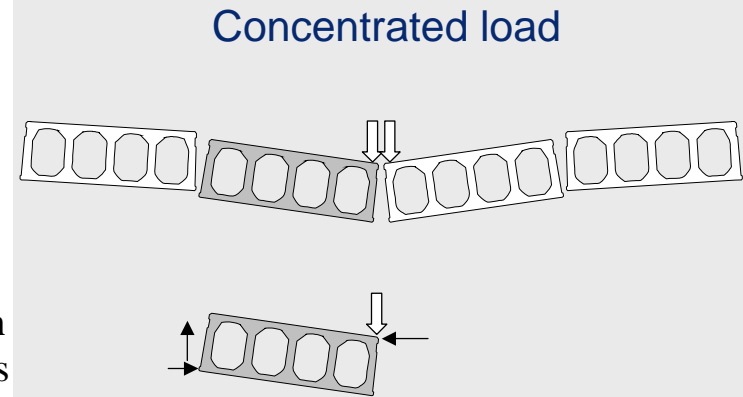
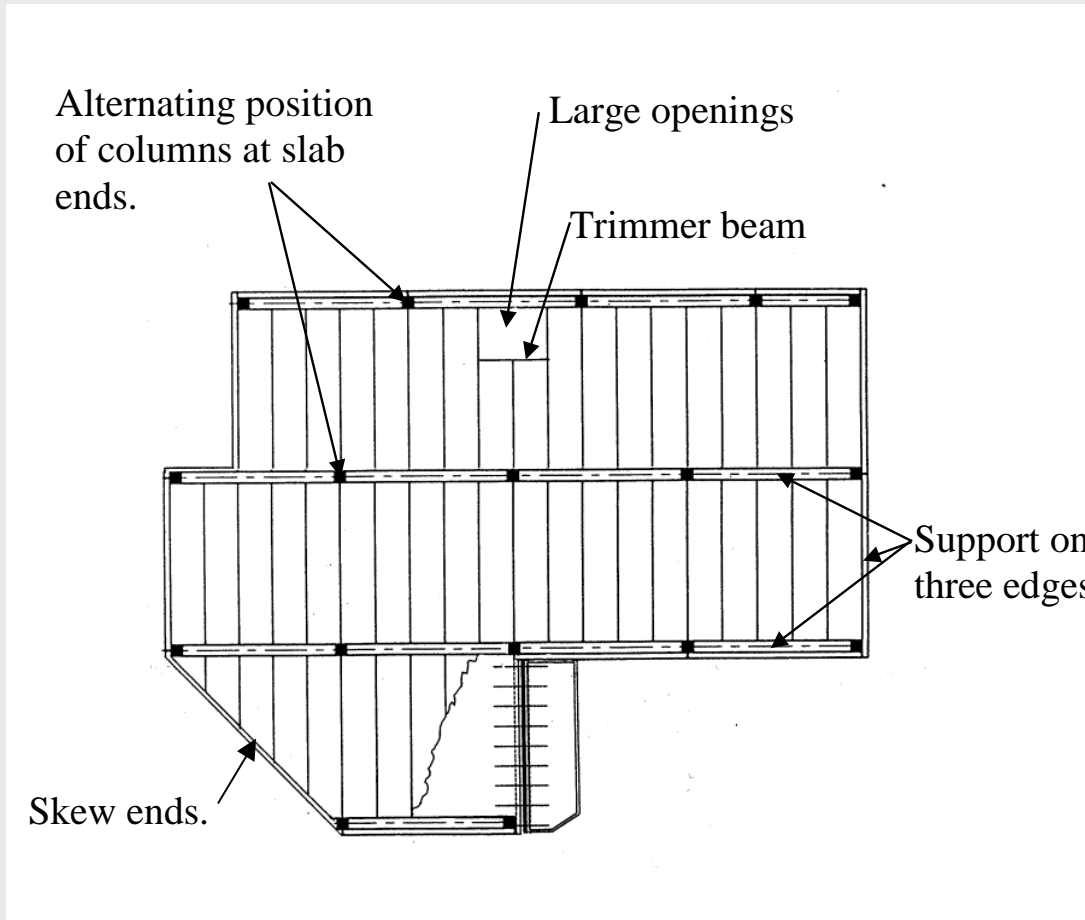
Project started January 1, 2002 and ended December 31, 2004

# Aim of the project

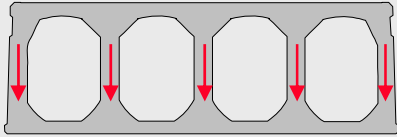
- To use the capacity of the hollow core slabs better
- To develop methods to design for combined shear and torsion in hollow core slabs
  - Single units
  - Whole floors



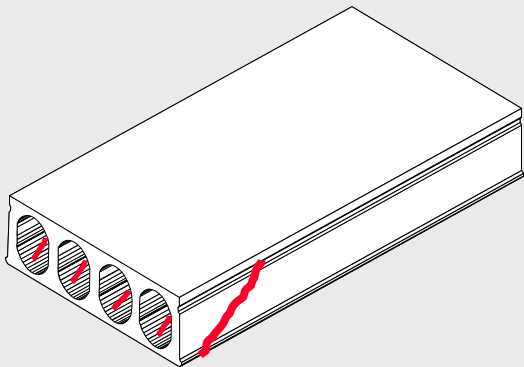
# Practical cases where torsion appears



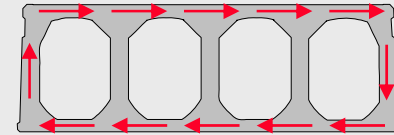
# Shear



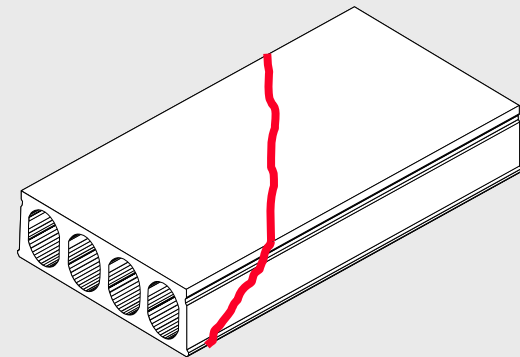
Web shear tension failure



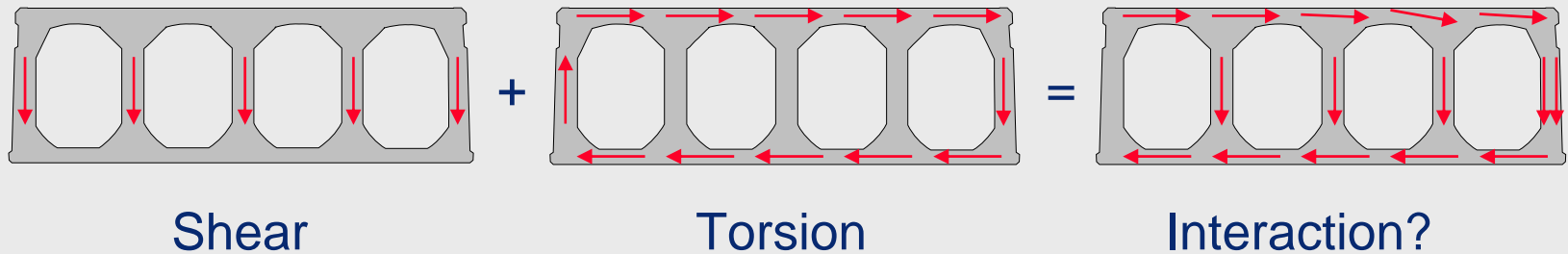
# Torsion



Torsion failure

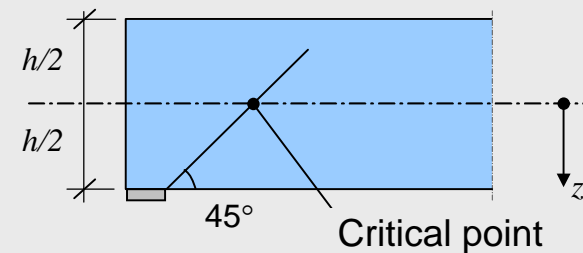
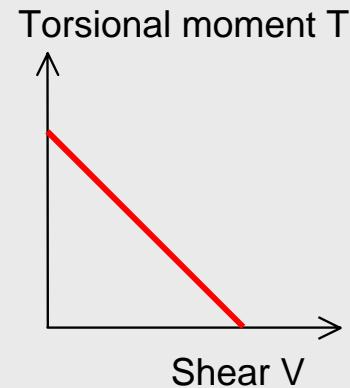


# Combined shear and torsion in a hollow core unit

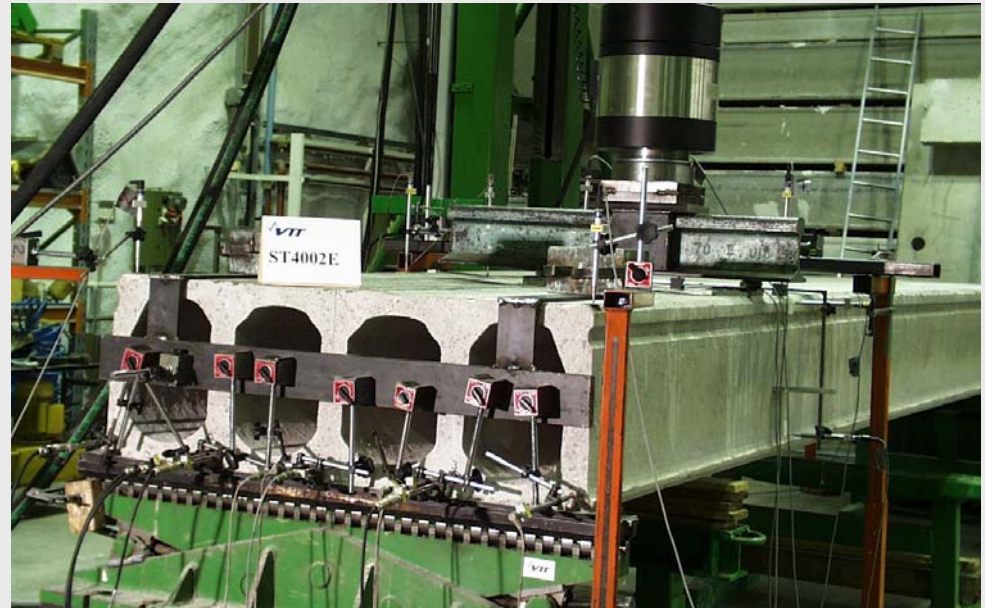
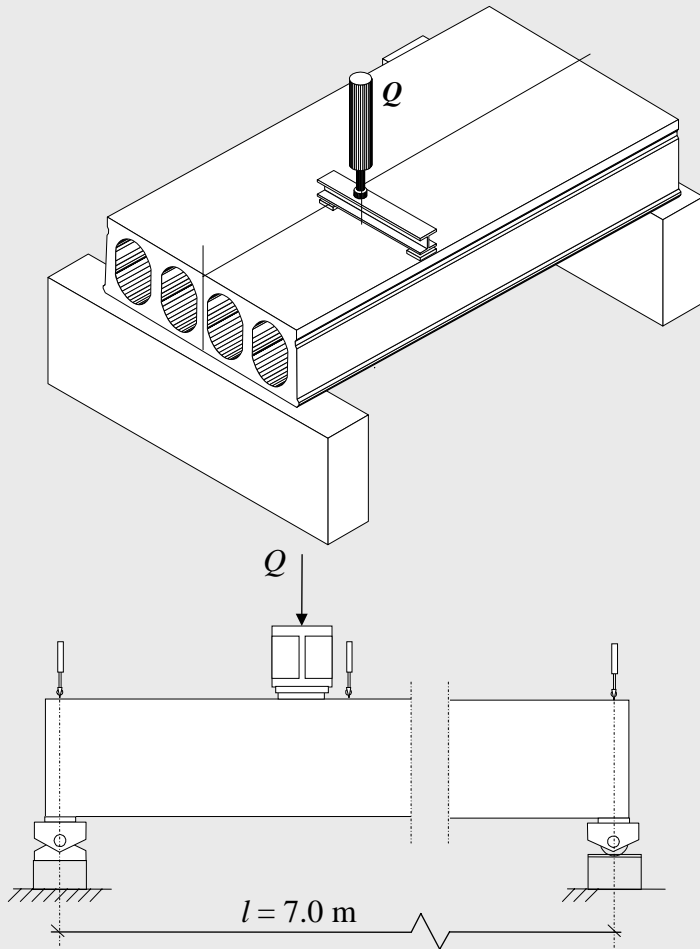


Design method used today (EN 1168):

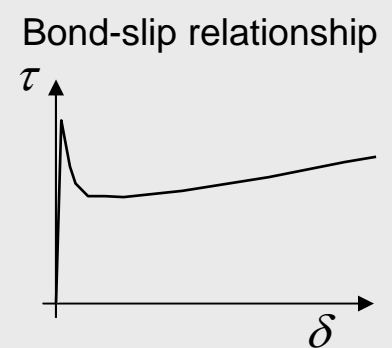
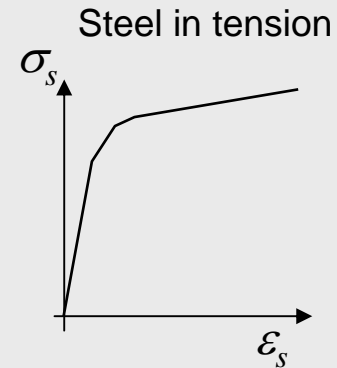
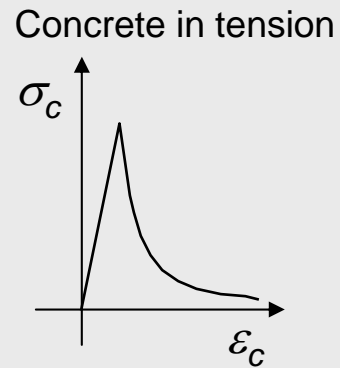
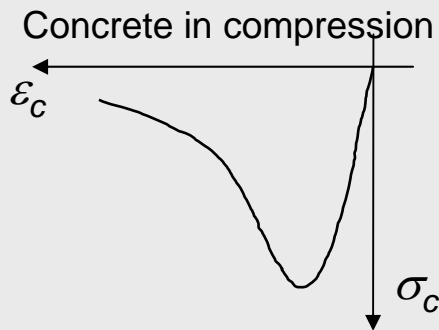
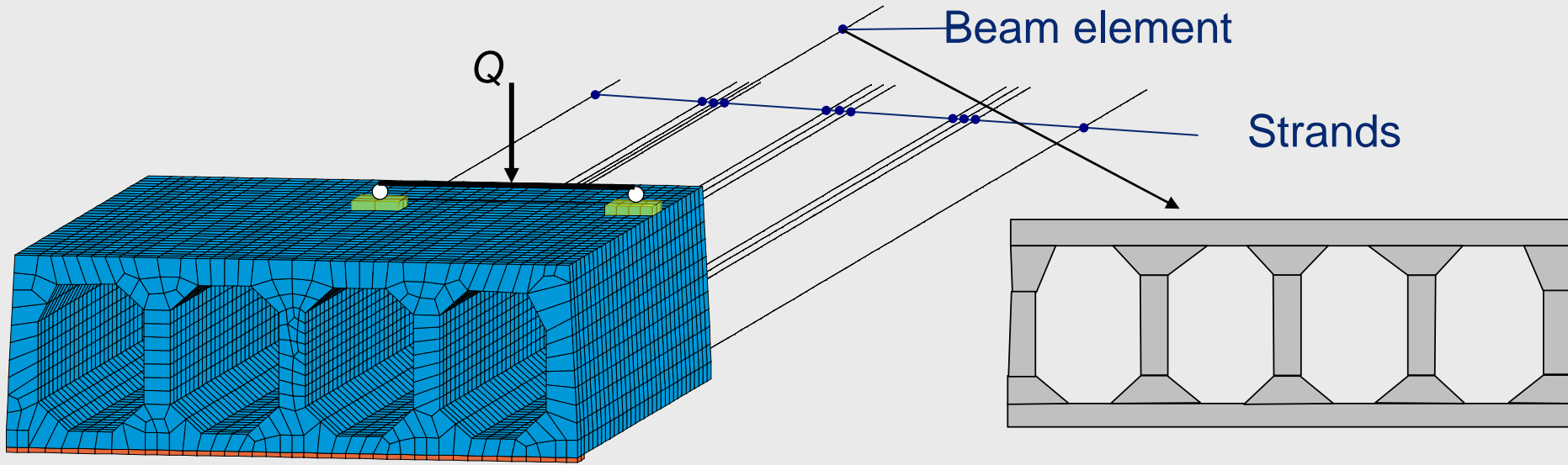
- Cracking means failure
- Only crack in web is considered
- One critical point
- Stresses are added linearly



# Tests on hollow core units loaded in shear and torsion

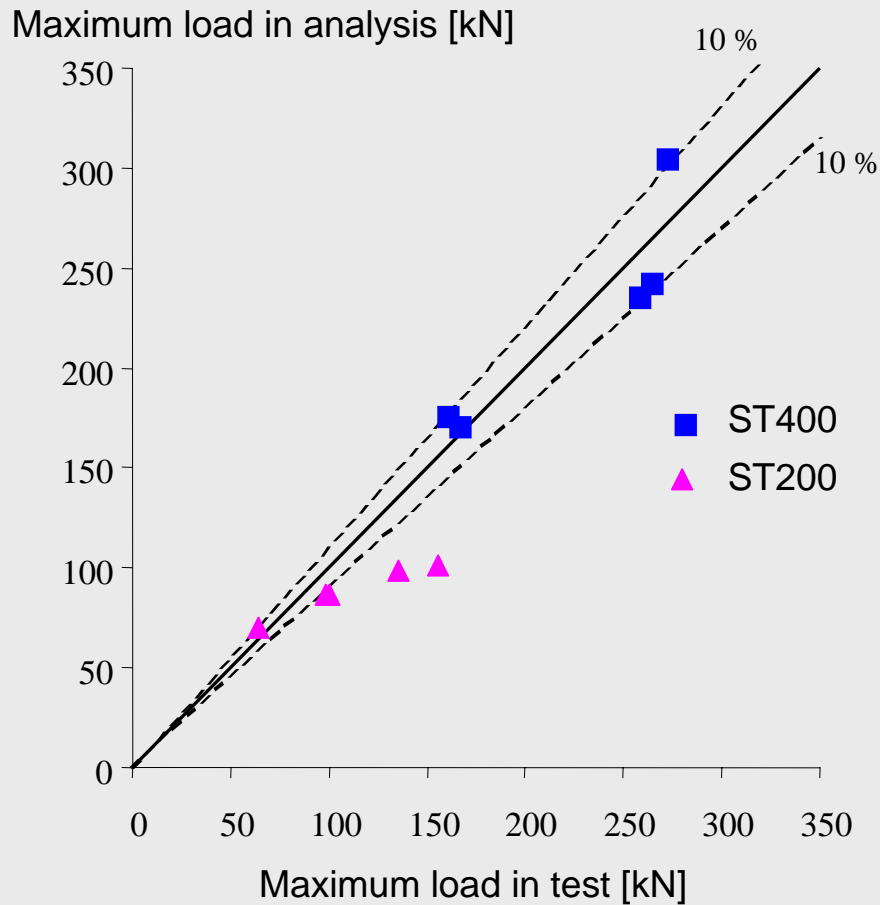


# FE model of hollow core unit

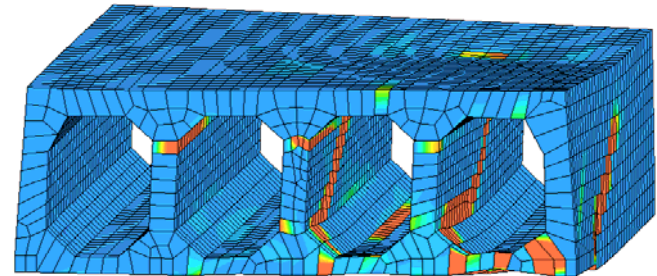
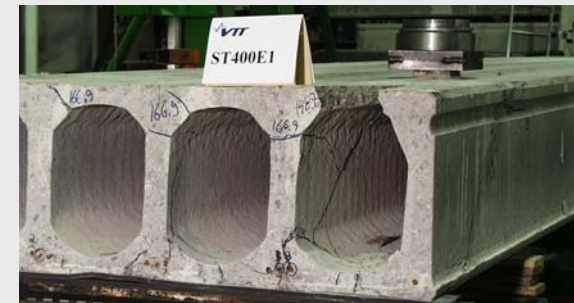




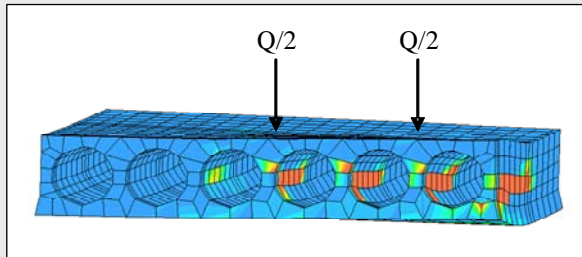
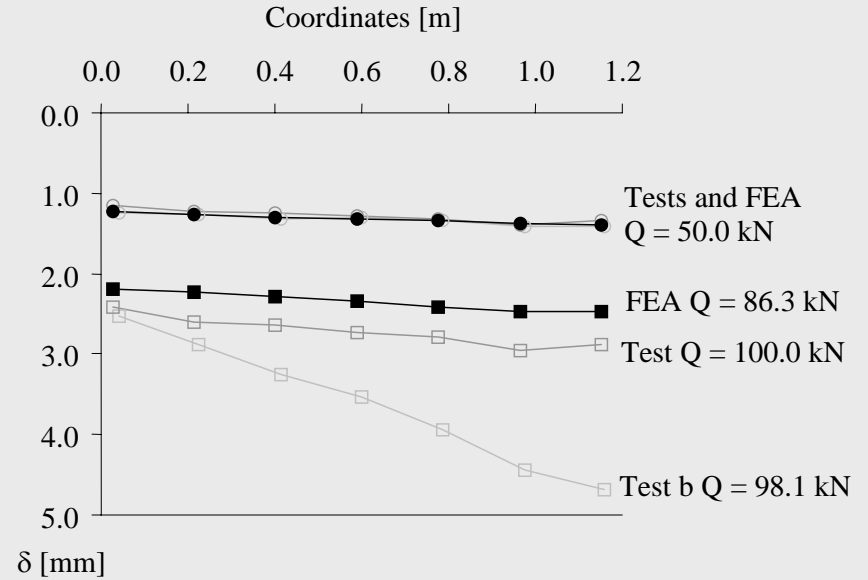
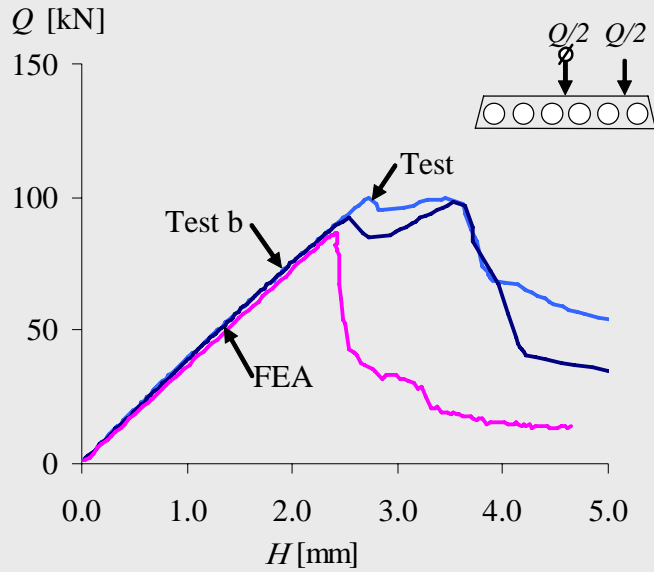
# Comparison of results



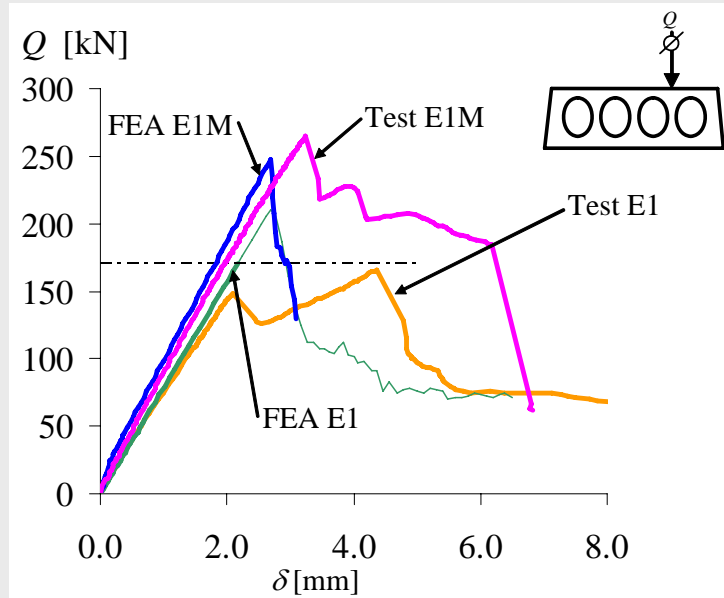
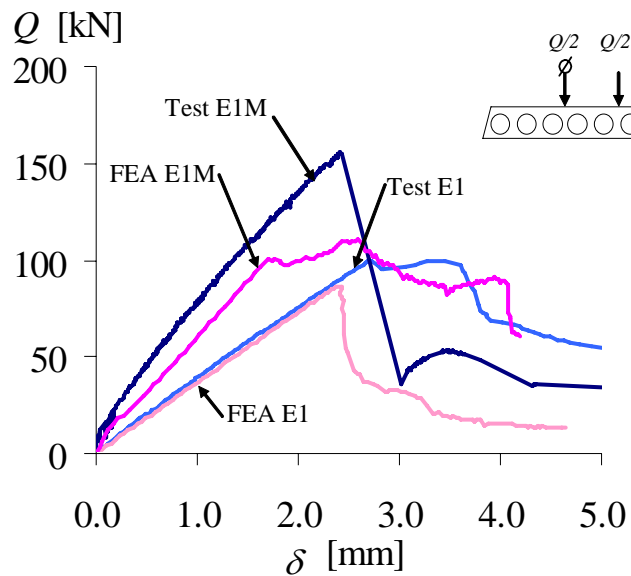
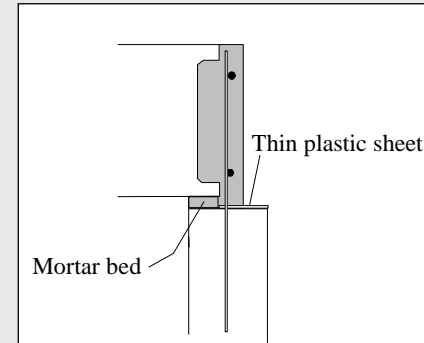
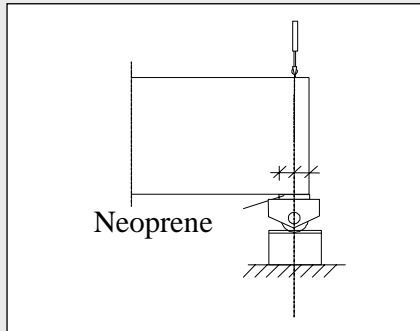
- Maximum load
- Load versus deflection
- Failure mode
- Crack pattern



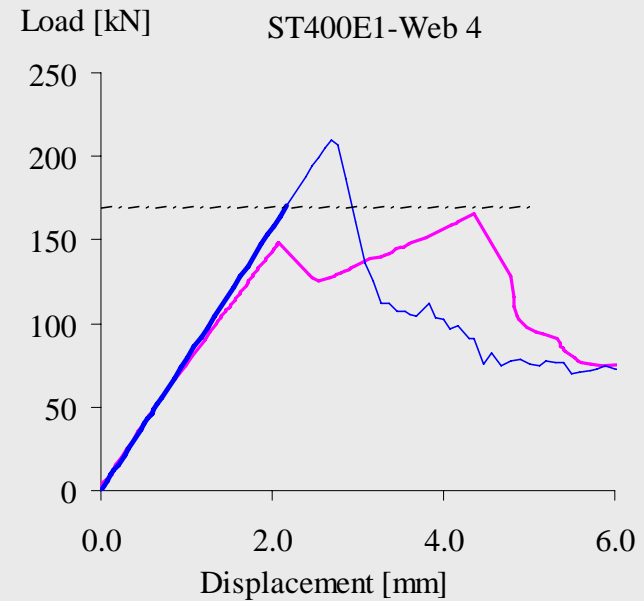
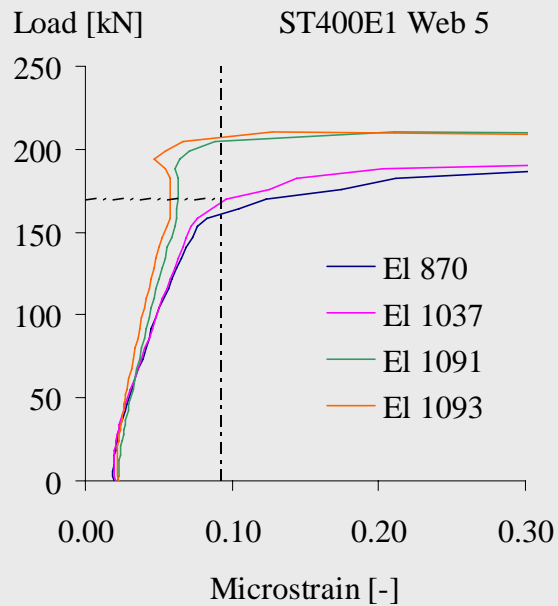
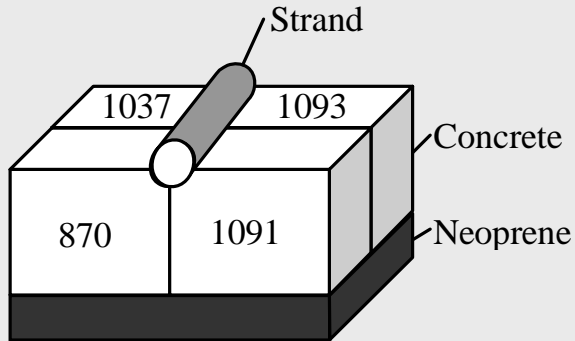
# Comparison of results



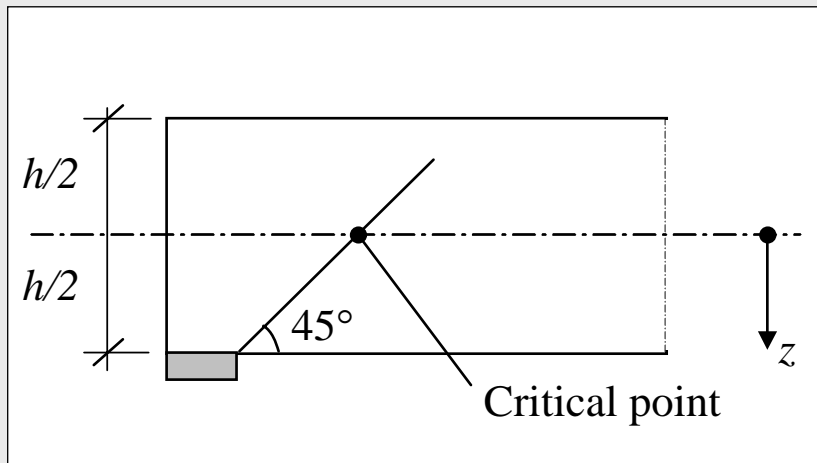
# Comparison of results



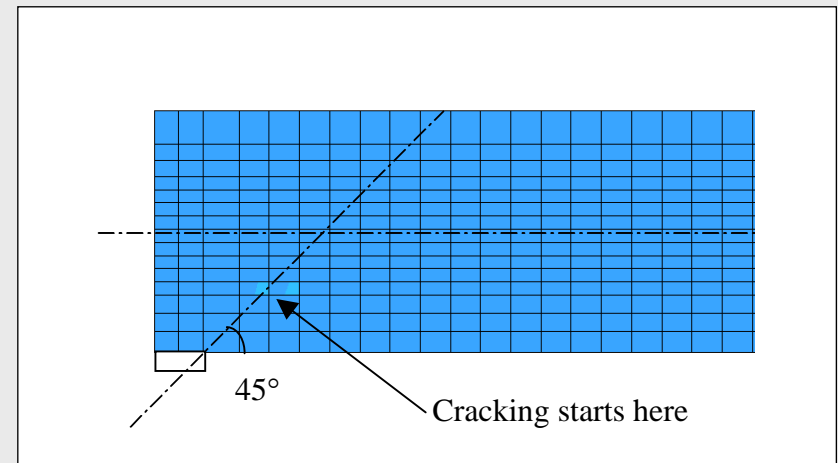
# Effect of neoprene



# Critical section for shear tension crack in 400 mm unit



Analytical model

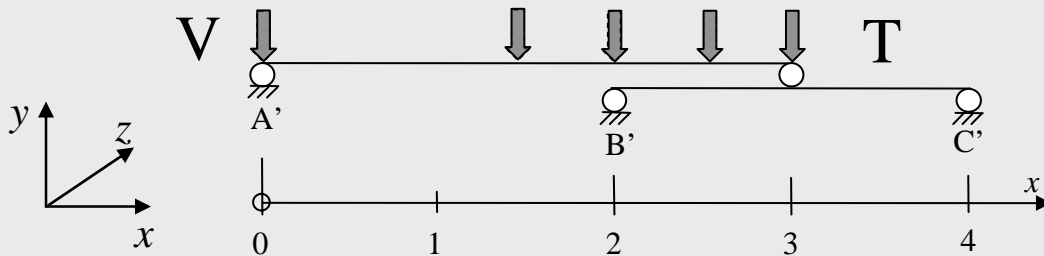
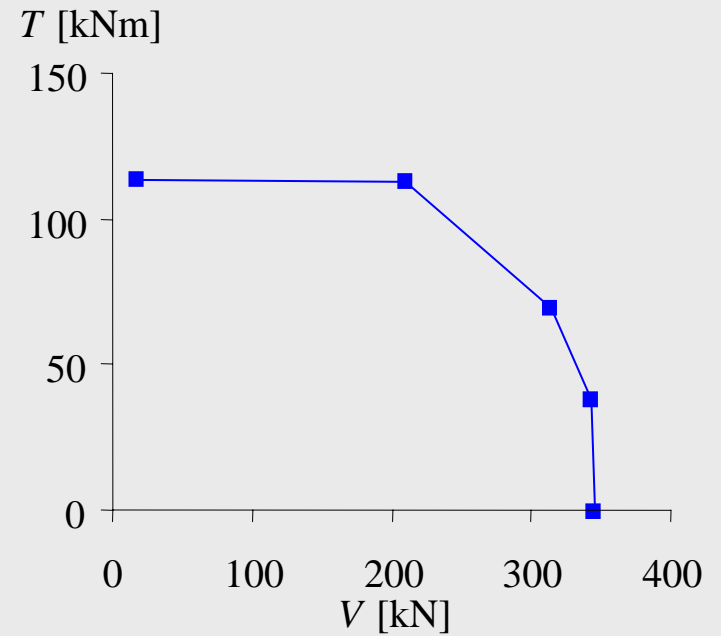
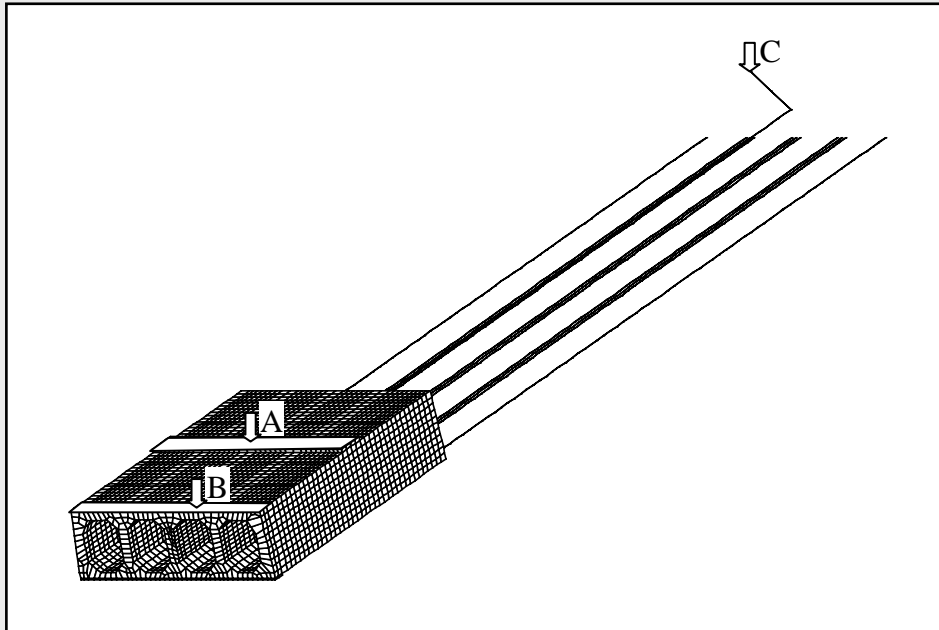


FE-analysis, pure shear

# Conclusions FE-analyses of HC-units

- FE-analyses of tests are able to capture the overall behaviour:
  - Failure mode
  - Maximum obtained load
  - Crack pattern
  - Vertical deflections (until first crack)
- Large difference in capacity due to support condition

# FE analyses to establish interaction diagram

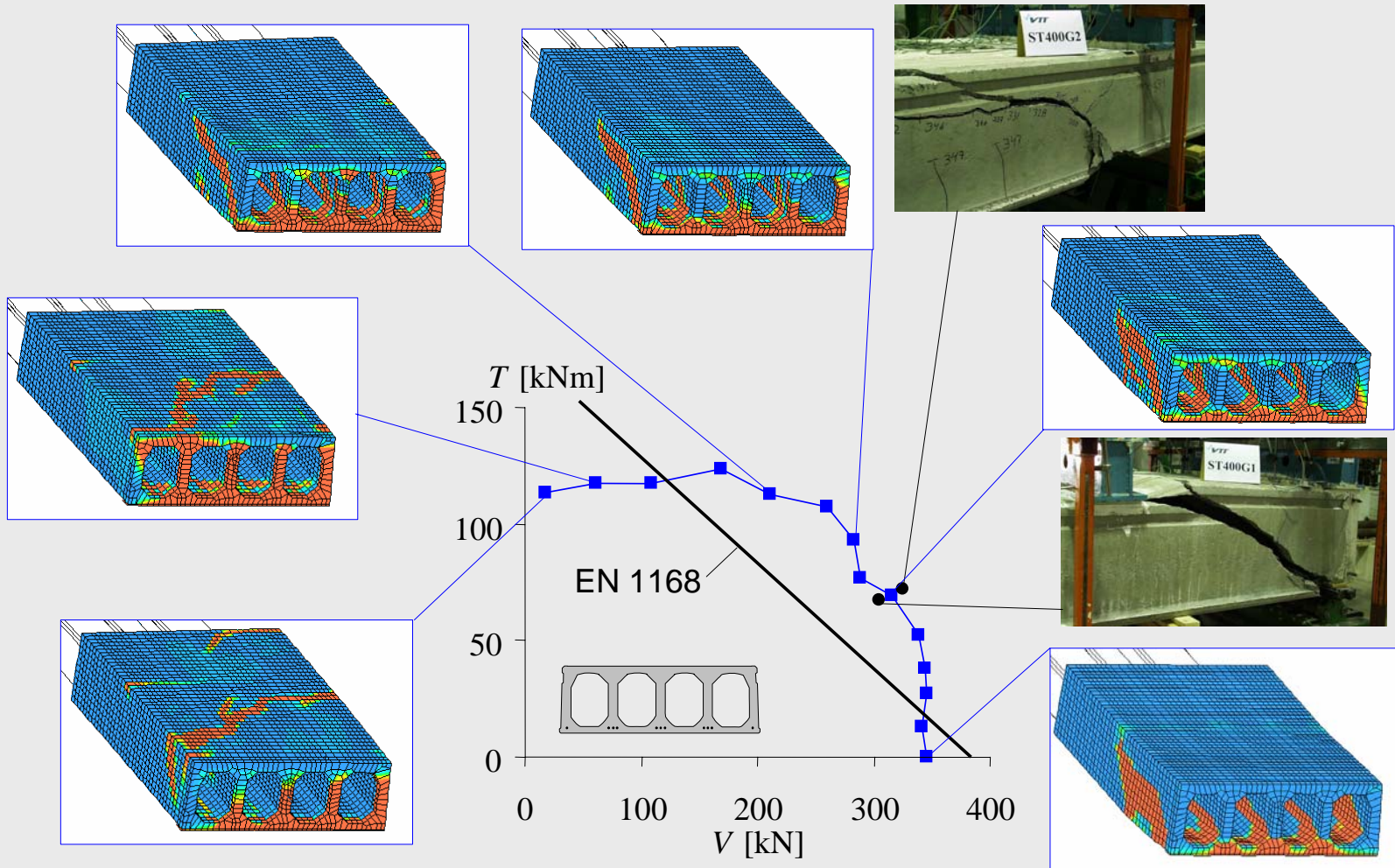


$$\delta_y^A = \delta_y^{A'}$$

$$\delta_y^B = \delta_y^{B'}$$

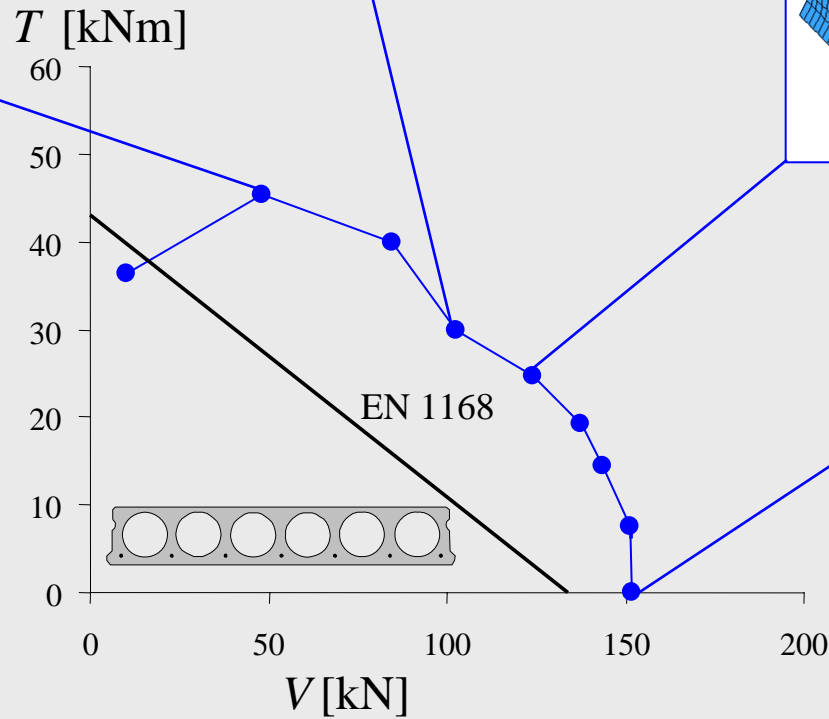
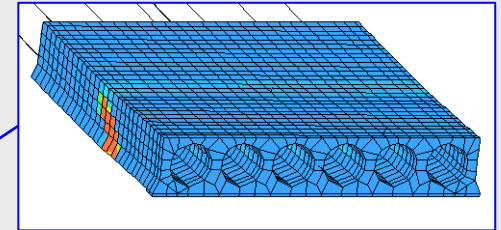
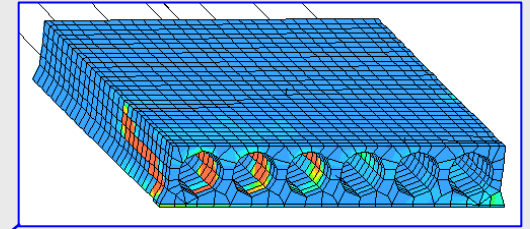
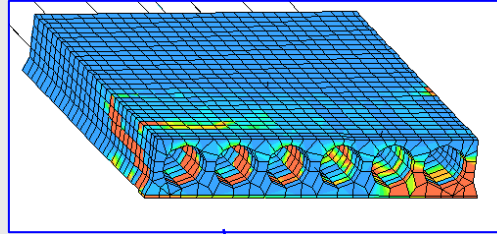
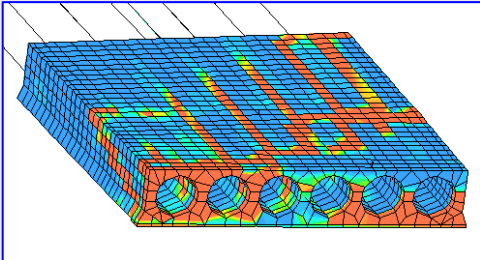
$$\delta_y^C = \delta_y^{C'}$$

# Interaction diagram for 400 mm unit

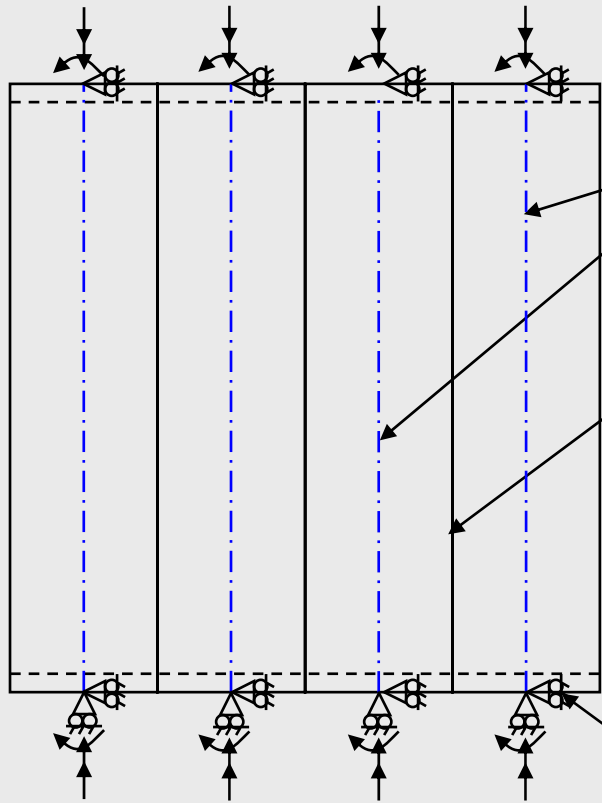




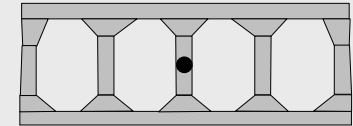
# Interaction diagram for 200 mm unit



# FE model of hollow core floor

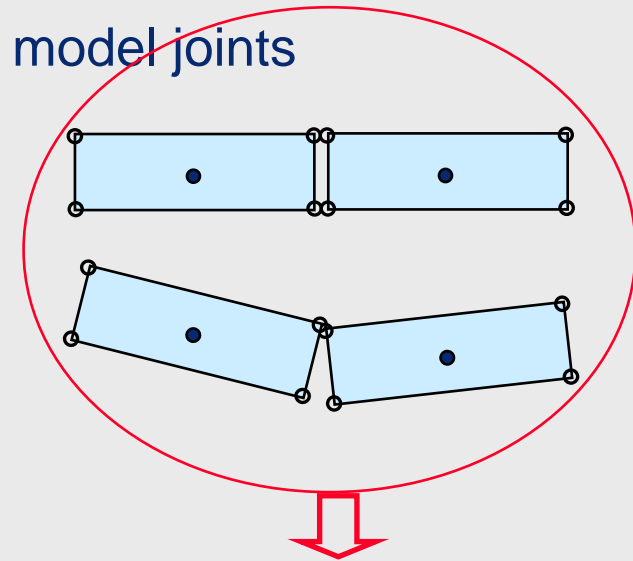


Beam elements to model single hollow core units



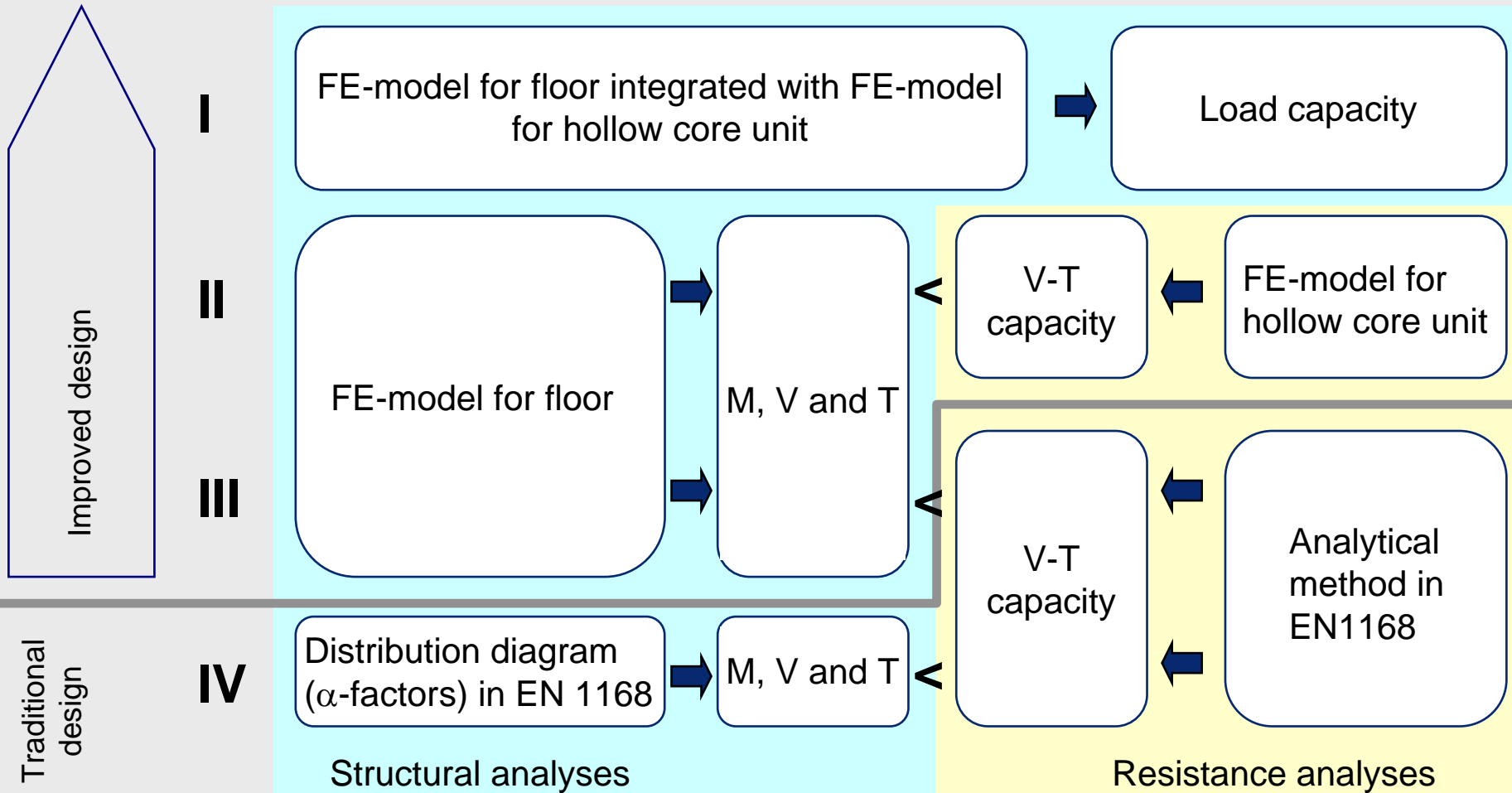
Interface elements to model joints

Tie beam modelled by tyings

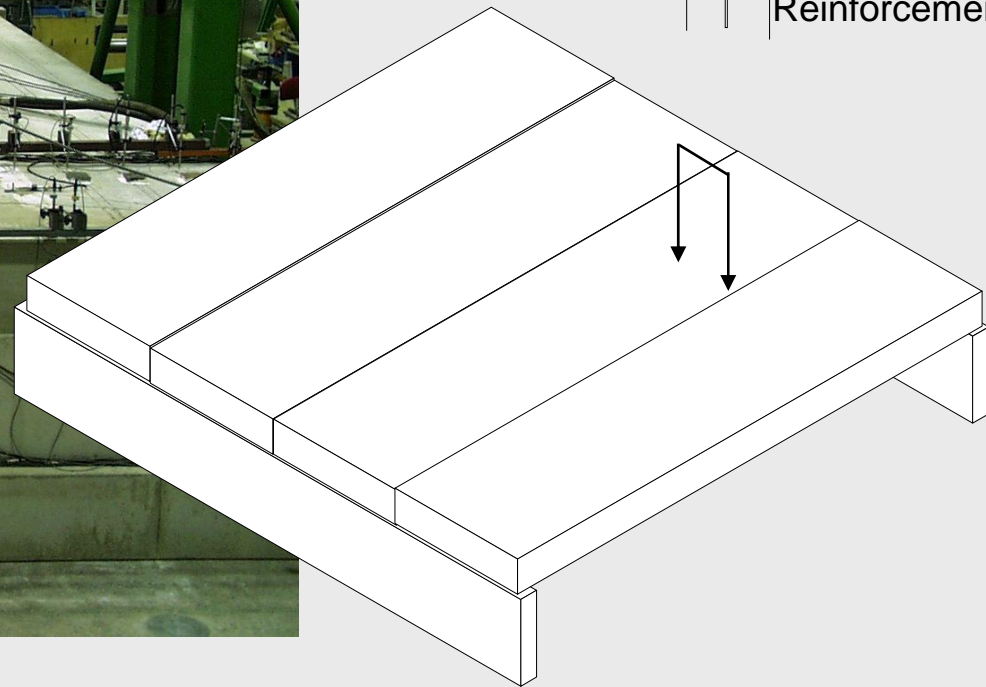
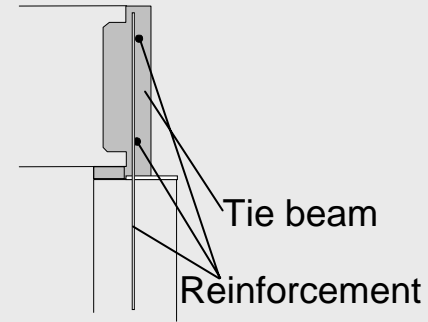
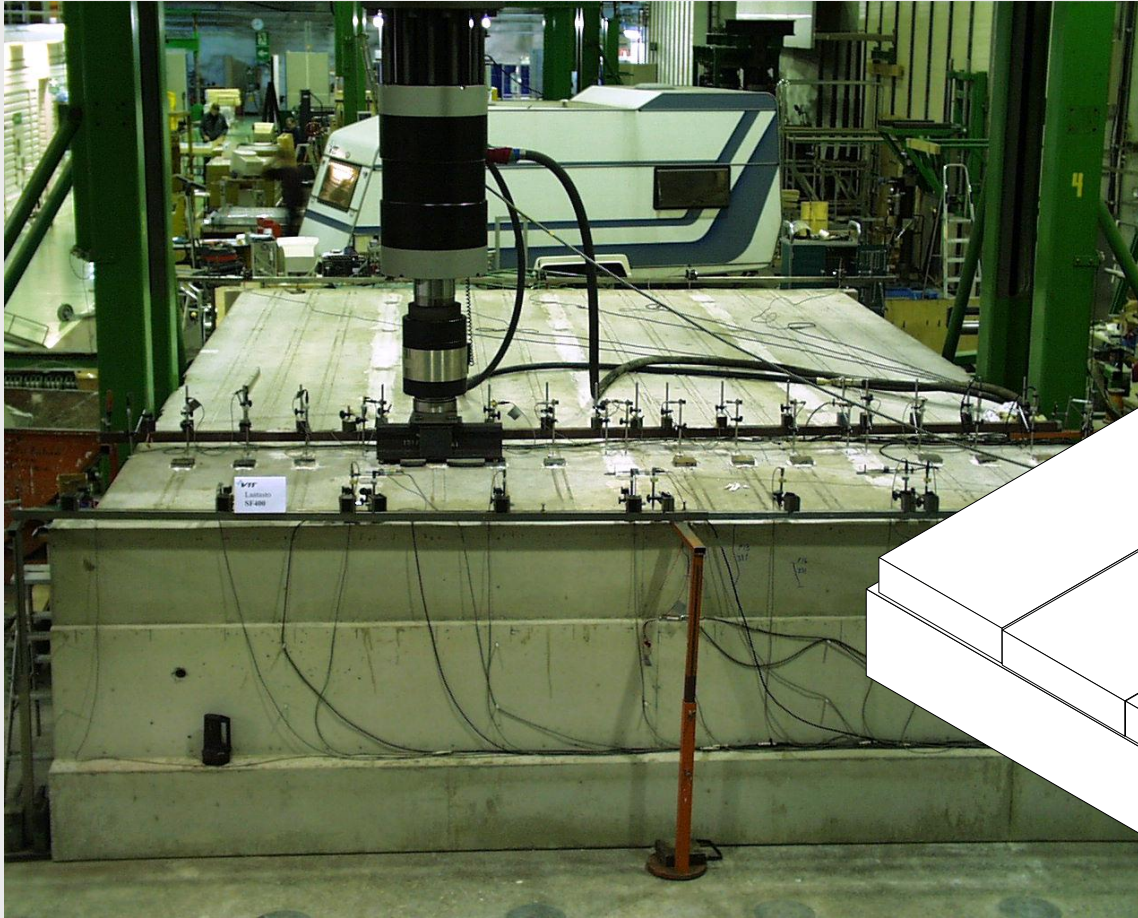


Reduced torsional moment

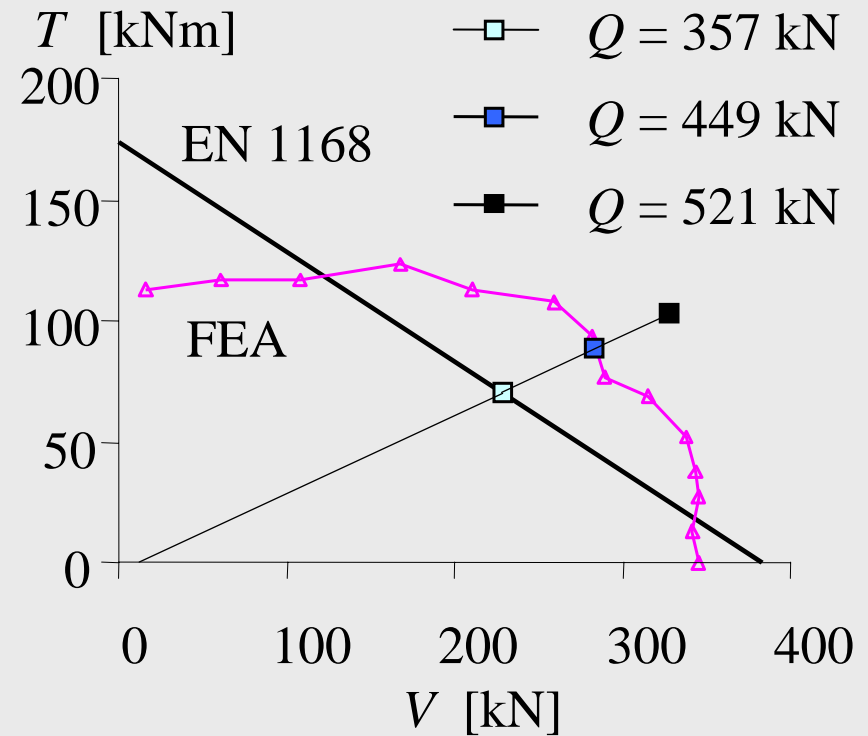
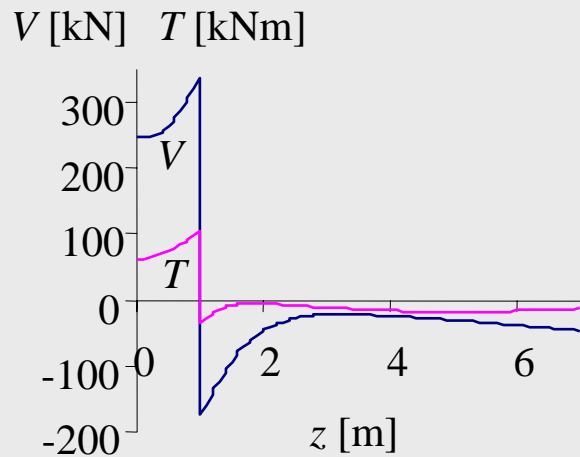
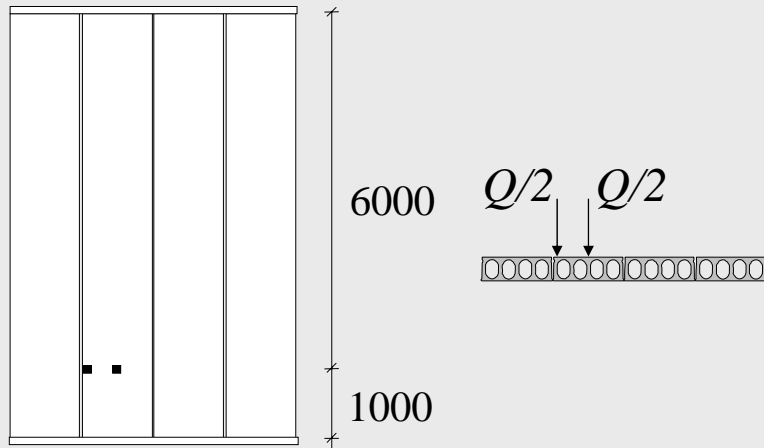
# Design of hollow core slabs



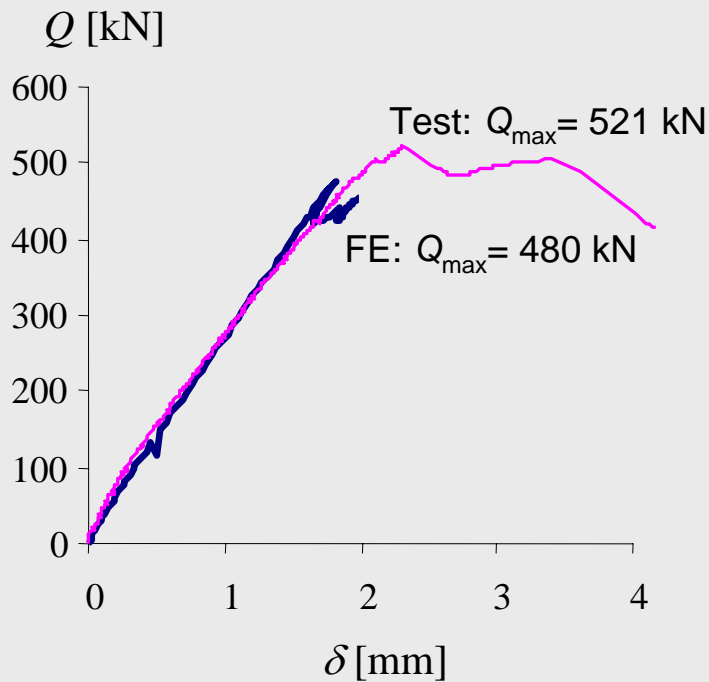
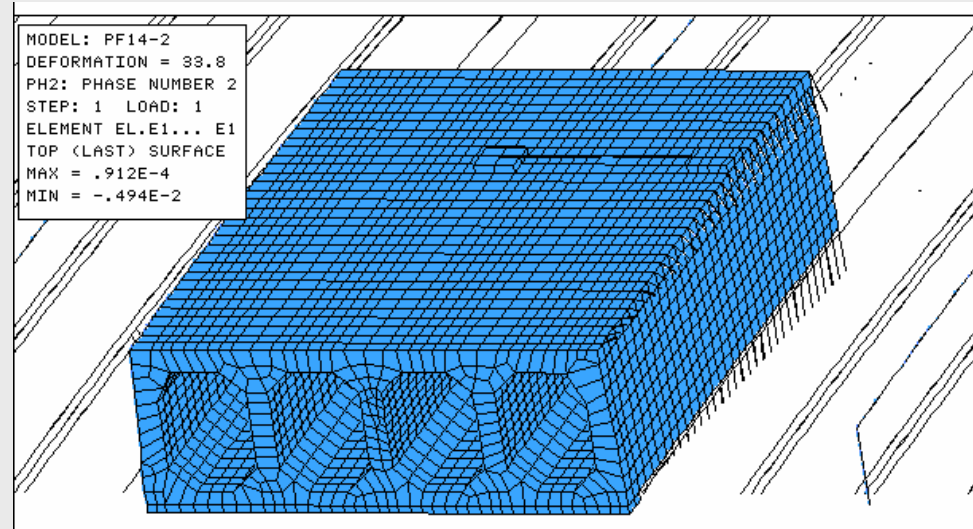
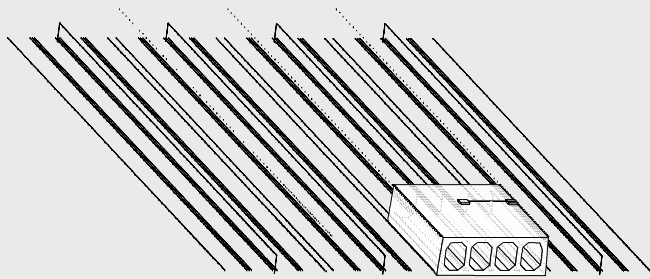
# Test on hollow core floor



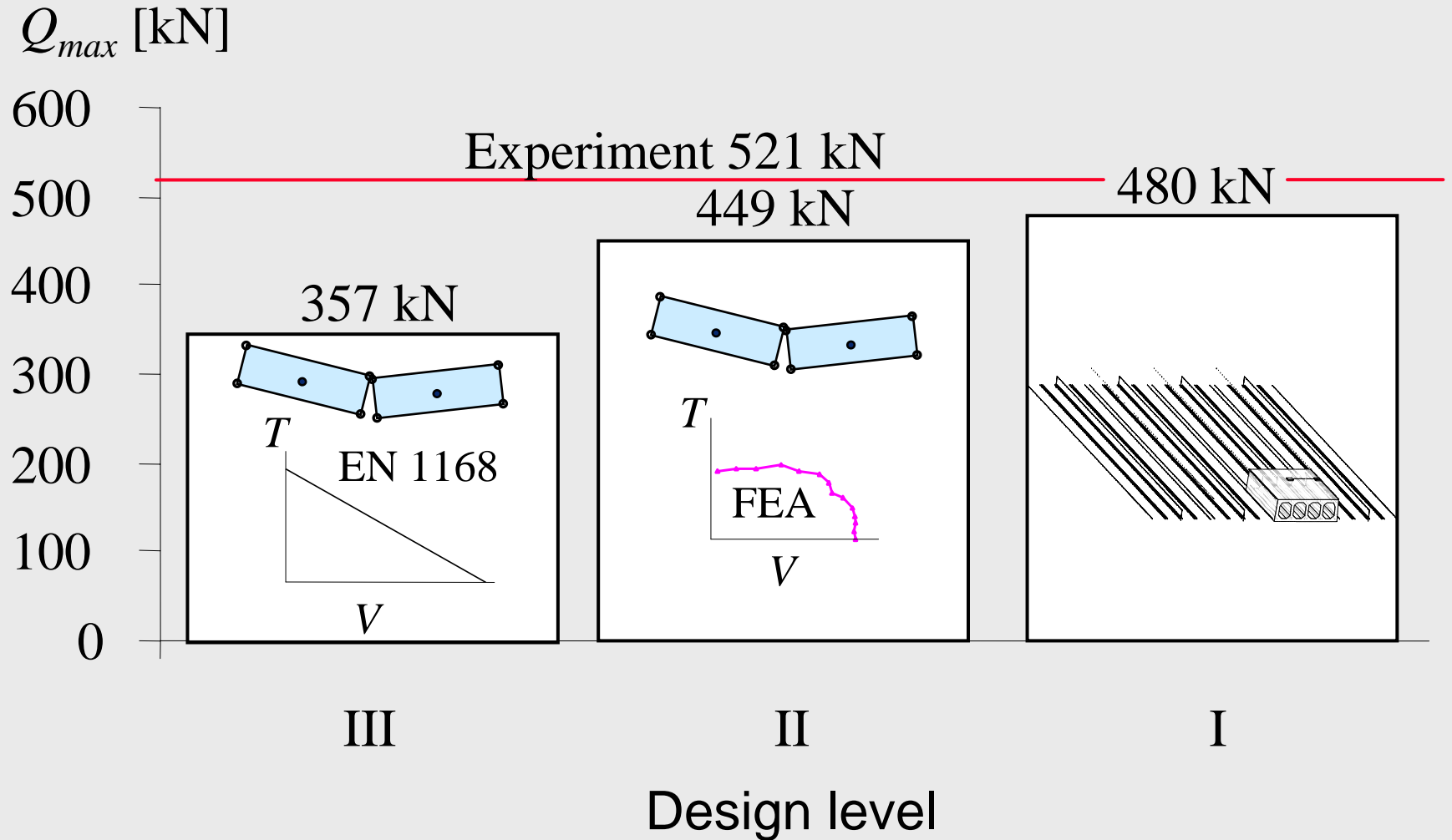
# Designing a floor – Design level III and II



# Simulation of a floor test – Design level I



# Floor, design example



# Conclusions

- Modelling methods for hollow core slabs were developed
  - Hollow core floor  $\Rightarrow$  Sectional forces  $M$ ,  $V$  and  $T$ 
    - Reduced torsional moment
    - Arbitrary geometries and loadings
  - Hollow core unit  $\Rightarrow$  Shear-torsion capacity
    - Higher resistance
- The capacity of the hollow core units can be used better



*Thanks!*

